Bamboo Engineering

Neil Thomas atelier one

Tall Timber





Woodland Trust











Date & Rime: Thu May 7 19:40:09 BST 2015 Position: +050.79502* / -002.67711 Athitude: 604ft Az muth/Bearing: 064 N64E 1138mills (True) Elevation Angle: +26.5 Horizon Angle: -00.3 Zoom: 1X

100







Bamboo



Green School Bali







Sharma Spring



Moon House

















Traditional dowelled
connections rely on loading a
small area of bamboo in shear.
This significantly restricts the
force that can be transferred.







140 / 110 110 / 110 110 / 80 PARALLEL STEPPED STEPPED 140 140 Ш П \Box 170 170 / 140 140/140 STEPPED PARALLEL CONNECTION NODE VARIANTS

L

110

П

110

Ш

VARIABLE PUTTY LAYERS FOR INTERMEDIATE TOLERANCE

80

110

140





3D printed coupler







"Full strength" cross node



24. notro for Ibuken

Concept design for an anticlastic bamboo gridshell, using principles inspired by Frei Otto, to form a great hall.





Green School Great Hall



Engineered Bamboo

Mechanical Properties - Engineered Bamboo Products

		Lamboo LVB	MOSO Bamboo N-finity	GL 24c	BauBuche	ReNuTeq VereLam	
Density	/ [kg/m ³]	705	650-700	365	800	673	
Young's Modu	lus (MOE) [GPa]	9.0	9.7	11	16.8	20.0	
Flexural Streng	th (MOR) [MPa]	77.2	56.7	24	75	88.3	
Compressive Strength [MPa]	Parallel	56.9	34.4	21.5	59.4	93.0	
	Perpendicular	19.8	9.9	2.5	14.8	20.9	
Tensile	Parallel	60.7 39.9		17	60.0	147.9	
[MPa]	Perpendicular	4.6	1.9 Edgewise 2.8 Flatwise	0.5	0.6	3.7	
Shear Stre	ength [Mpa]	20.0	4.6 Edgewise 6.5 Flatwise	3.5	4.5	20.0	









CURRENT FLOOR SYSTEM

PROPOSED FLOOR SYSTEM (OPTION 1)



Japanese Joinery Examples





2. Design Parameters

The following design parameters have been established for the purposes of the cross-comparison exercise.

2.1 Court Layout

The structure is to accommodate 2 basketball courts. The arrangement of these courts is still undecided. The below arrangement has been selected for the





1. INTRODUCTION

This document compares various structural forms proposed for the project - Fort Erie Basketball Court.

Stability in lengthwise direction is not yet addressed and thus will require further bracing.



3. Comparison Exercise

3.3 Barrel Arch – Propped and Released

3.3.4.2 Deflection cases



- 3. Comparison Exercise
- 3.4 Portal
- 3.4.4.2 Deflection Cases



- 3. Comparison Exercise
- 3.5 Pitched Roof
- 3.5.4.2 Deflection Cases



3. Comparison Exercise

3.6 Domed Roof – Pinned Internal Dome A

3.6.4.2 Deflection Cases

22 2 25 10

13

49mm



42mm

4. Comparison Summary

Scenario	1. Barrel Ar	ch – No Prop	2. Barrel Arch – Prop			3. Barrel	Arch – Pro Release	pped and d		4. Portal		5. Pitched Roof			
	1a. Deflection Limit: L/200														
Section	Arch 1000 x 300 U: 0.54	Arch 800 x 200 U: 0.98	Arch	800 x 200	U:0.97	Arch	850 x 200	U:0.93	Beams	1100 x 200	U: 0.79	Top Chord	600 x 200	U: 0.76	
01203			Column	400 x 200	U:0.38	Column	400 x 200	U:0.98	Columns	1100 x 200	U:0.98	Bottom Chord	400 x 200	U: 0.98	
							Back Props 400 x 200 U:0.95					Truss Members	200 x 200	U: 0.24	
												Tie	89.3 CHS	U: 0.86	
												Columns	800 x 200	U: 0.96	
Total Volume	231 m ³	117 m ³	128 m ³			133 m ³			144 m ³			173 m ³			
Deflection Envelope	85mm L/211	309mm L/58.25	104mm L/200			129mm L/162			93mm L/354			132mm L/250			
Additional notes	 Effective length = 0.85L All joints fixed 	tive length = 0.85L • Effective length = 0.85L ints fixed • All joints fixed		 Effective length = 0.85L Arch joints fixed; column joint pinned 			 Effective length = 0.85L Arch joints fixed; column joint pinned, back prop joint pinned 			 Effective length = 0.85L Lateral restraints taken at 2.3m centres All joints fixed 			Effective length = L All joints pinned Lateral restraints for top chord taken at 2.3m centres		

4. Comparison Summary

Scenario		6. P	inned Domes	7. Fixed Domes						8. Fixed Domed – Internal Props								
	6a. Arrangement A 6b. Arrangement B					7a. Arrangement A			7b. Arrangement B			8a. Arrangement A			8b. Arrangement B			
Section Sizes	CENTRAL ARCH	800 X 200	U:0.82	CENTRAL ARCH	950 X 250	U:0.91	CENTRAL ARCH	900 x 250	U:0.79	CENTRAL ARCH	900 x 250	0.88	CENTRAL ARCH	250 X 200	0.33	CENTRAL ARCH	250 X 200	0.24
	INTERNAL DOME STRUCTURE	350 X 250	U:0.93	INTERNAL DOME STRUCTURE	300 x 250	U:0.93	INTERNAL DOME STRUCTURE	250 X 250	U:0.75	INTERNAL DOME STRUCTURE	250 X 250	0.98	INTERNAL DOME STRUCTURE	250 X 200	0.75	INTERNAL DOME STRUCTURE	250 X 200	0.74
	RING BEAM	200 X 200	U:0.43	RING BEAM	200 x 200	U:0.49	RING BEAM	200 X 200	U:0.23	RING BEAM	200 X 200	0.49	RING BEAM	200 X 200	0.23	RING BEAM	200 X 200	0.24
	CENTRAL COLUMN	800 X 250	U:0.81	CENTRAL COLUMN	600 x 200	U:0.83	CENTRAL COLUMN	600 x 200	U:0.74	CENTRAL COLUMN	600 x 200	0.9	CENTRAL COLUMN	300 X 300	0.28	CENTRAL COLUMN	300 X 300	0.40
	PERIMETER COLUMNS	400 X 200	U:0.34	PERIMETER COLUMNS	400 x 200	U:0.4	PERIMETER COLUMNS	400 x 200	U:0.32	PERIMETER COLUMNS	400 x 200	0.38	PERIMETER COLUMNS	400 X 200	0.98	PERIMETER COLUMNS	400 X 200	0.82
Total Volume	Internal Dome – 133 m ³ Central Arch – 5 m ³ Ring Beam – 6.25 m ³ Columns - 27 m ³			Internal Dome - 127 m ³ Central Arch - 5 m ³ Ring Beam - 7 m ³ Columns - 34 m ³			Internal Dome – 95 m ³ Central Arch - 5m ³ Ring Beam – 6.25 m ³ Columns – 29.5 m ³			Internal Dome - 105 m ³ Central Arch - 5 m ³ Ring Beam - 7 m ³ Columns - 34 m ³			Internal Dome - 78 m³ Ring Beam – 6.25 m³ Columns - 31 m³			Internal Dome – 92.7 m ³ Ring Beam - 7 m ³ Columns - 38 m ³		
Deflection	n 53mm			54mm			53mm			42mm			42mm			47mm		
Additional	Effective length = L Internal dome joints are pinned			Effective length Internal dome j	th = L joints are pinned i bit = L joints are pinned i bit = L i bit = L			ixed ch pinned n pinned	Effective length = 0.7L All internal dome joints fixed Connection to central arch pinned Connection to ring beam pinned			Effective length = 0.7L All internal dome joints fixed Connection to columns fixed Connection to ring beam pinned			Effective length = 0.7L All internal dome joints fixed Connection to columns fixed Connection to ring beam pinned			















